

S. K. GHOSH ASSOCIATES LLC Seismic and Building Code Consulting





Version 1.8.0

User Manual

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DISCLAIMER

Every attempt has been made to ensure correctness in implementing code provisions as well as the accuracy of the calculations in RTWall. In using the program, however, the user accepts and understands that no warranty is expressed or implied by SKGA as to the accuracy or the reliability of the program. The user must carefully read this manual and thoroughly understand the assumptions of the program and must independently verify the results. In addition, in no event shall SKGA, or its employees or affiliates be liable for any indirect, incidental, consequential, or punitive damages whatsoever relating to the use of RTWall.

REVISION HISTORY

Version 1.8.0 (Release Date: May 8, 2025)

- 1. Correction is made in the way flexural strength of a masonry stem is determined for ASD.
- 2. Revisions are made to determine reduction in flexural strength due to partial bar development in the stem.
- 3. Revisions are made in the way an input file is read to address error messages in specific situations.

Version 1.7.0 (Release Date: April 11, 2023)

- 1. Support for the 2021 IBC and ACI 318-19 is added.
- 2. Improvements in the output report, including in the stem moment and shear diagrams, are made.
- 3. Corrections made in the way shear strength of masonry stem is determined.
- 4. Corrections made in the way effective depth, d, is determined for concrete stem in certain situations.

Version 1.6.1 (Release Date: February 7, 2023)

1. Minor updates in the way the program communicates with the license server.

Version 1.6.0 (Release Date: October 7, 2020)

1. Support for the 2018 IBC and 2016 TMS 402 is added.

Version 1.5.0 (Release Date: July 30, 2018)

1. An error was corrected where the program was not calculating the correct restraint reactions for seismic force when the seismic force was entered directly from a geotechnical report.

Version 1.4.0 (Release Date: August 30, 2017)

- 1. Major revisions incorporated in the way the program handles restrained walls.
- 2. Some errors are corrected in the calculation of shear and bending moments in the stem from an adjacent strip, line and point loading.
- 3. Output is now generated in PDF format for easy navigation and file handling.
- 4. A feature is added for easy renewal of program license.

Version 1.3.0 (Release Date: February 1, 2017)

1. Support for the 2015 IBC, ACI 318-14 and 2013 TMS 402 is added.

Version 1.2.1 (Release Date: September 27, 2016)

1. An error has been fixed where the design shear force and bending moment in the stem due to adjacent strip, line and point loads were not being initialized properly.

Version 1.2.0 (Release Date: June 20, 2016)

1. Design of restrained (basement) walls with up to five restraints along the height of the stem has been added.

Version 1.1.1 (Release Date: June 13, 2016)

1. An option to specify embedment type (into the footing) for stem's vertical bars has been provided.

Version 1.1.0 (Release Date: April 7, 2016)

- 1. Design of CMU cantilever retaining walls based on both Allowable Stress Design and Strength Design requirements of MSJC has been added.
- 2. A new option for user-defined lateral pressure on the shear key has been added.
- 3. Program interface has been graphically improved.







USER INTERFACE AT A GLANCE





INPUT FIELDS

The input fields in the RTWall interface are mostly self-explanatory. However, a short description of each input field is provided below for better clarity. Different input fields are marked by item numbers, as shown below in Figure 1. Units used in this program are as follows:

BASICS

| Ĩ | Basics | Geometry | Materials | Load | s Reinfor | rcement |
|----|------------|---------------------------------|--|----------|---------------------|----------|
| | Desig | n Criteria | | | | |
| | Code | used | | 20 | 021 IBC 🔒 🔒 | ~ |
| | F.S. (w | ı/o seismic) | ? | 1.3 | 5 2 | |
| I | F.S. (w | ı/ seismic) | ? | 1. | 1 3 | |
| | Desig | n Assumpti | ons | | | |
| 4 | w 🗌 | all is restrai | ned against | sliding | ? | |
| 5 | 🗹 Fa | ictor soil we | ights as dea | d load | instead of I | H load |
| 6 | ⊠ Us | se vertical co ability check | omponent of (s | active | e pressure in | |
| 7 | ⊠ Us | se vertical co ability check | omponent of (s | f surcha | arge load in | |
| 8 | D D | on't combin iction force | e passive protocological to resist slid | essure | resultant fo | rce with |
| 9 | - Ne | eglect soil o | ver toe for t | oe des | ign 🤉 | |
| 10 | 🗌 Ne | eglect beari | ng at heel fo | r heel (| design _? | |
| 11 | Ve Ve | ertical bars a | re develope | d at th | e top of ste | m ? |
| | Projec | t Informati | on | | | |
| 1 | 2 Projec | ct Title | | | | |
| 1. | Prepa | red by | | | | |
| 14 | 4 Comp | bany | | | | |
| 1 | 5 Phone | e [| | | | |
| 1 | 6 Email | · [| | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |





Item 1: Select the code version to follow. Options are: 2009 IBC through 2021 IBC.

Item 2: Safety factor for retaining wall sliding and overturning when earthquake loads are not included. Minimum code prescribed value is 1.5.

Item 3: Safety factor for retaining wall sliding and overturning when earthquake loads are included. Minimum code prescribed value is 1.1.

Item 4: Select if wall is restrained against sliding. If selected, the program will skip the stability check against sliding.

Item 5: Select if the load factor on soil weights must be the same as that on dead load (D). If not selected, the program will factor soil weights as earth pressure (H).

Item 6: Select if vertical component of active pressure can contribute to wall's stability.

Item 7: Select if vertical component of surcharge load can contribute to wall's stability.

Item 8: Select if only the larger of passive pressure resultant force and friction force is used to resist sliding. If not selected, the combination of both forces will be used.

Item 9: Select if weight of the soil over toe must be neglected in structural design of the toe.

Item 10: Select if upward bearing pressure below heel must be neglected in structural design of the heel.

Item 11: Select if vertical bars are developed at the top of the stem. If not selected, concentrated shear and moment cannot be applied at the top of the stem.

Items 12 through 16: *Project Info* fields can be used to enter the details of the anchor design project. These details are displayed in the header of the output reports.



GEOMETRY

| Basics | Geometry | Materials | Loads | Reinforce | ment |
|--------------------|------------------|-------------------------|-------------|-------------------|-------|
| Backf | ill | | | | |
| Heigh | it from top o | f footing | | 7 1 | ft |
| Slope | (α) | | | 10 2 | deg |
| <mark>3</mark> ⊿ w | ater table | | height | 2 4 | ft |
| -Soil O | ver Toe | | | | |
| Heigh | it from top o | f footing | | 3 5 | ft |
| Depth | n of soil over | toe to igno | re ? | 1 6 | ft |
| Stem | | | - | | • • |
| Heigh | it from top o | f footing | | 9 7 | ft |
| Wall t | type | <mark>8</mark> R | einforced | I Concrete | ~ |
| Thichr | ness at top | | | 12 9a | in. |
| Extra | thickness at I | bottom hee | l-side | 0 <u>10a</u> | in. |
| Extra | thickness at | bottom toe | side | 0 <u>11a</u> | in. |
| 12 ☑ W | /all is restrain | ied <u>(EDIT)</u> | | | |
| - Footi | ng | | | | |
| Thickr | ness | | | 24 13 | in. |
| Heel | width 4 | 14 ft To | oe width | 4 15 | ft |
| 16 Sh | ear key prov | vided | | | |
| Width | 12 17 | in. | Depth | 12 18 | in. |
| Positi | on Under ste | em | 19 | ~ <u>20</u> | ft |
| 21 Aj | oply passive | pressure on | ly to the l | cey ? | |
| 22 Ad | djust active-s | ide pressure | e on the k | ey ? | |
| Тс | op 2 | <mark>3</mark> psf/ft E | Bottom | <mark>24</mark> p | sf/ft |

Figure 2. GEOMETRY input page of RTWall

- Item 1: Height of the backfill soil (ft) measured from top of footing.
- Item 2: Slope of the backfill soil (degree).
- Item 3: Select if water table exists in the backfill soil.
- Item 4: height of the water table (ft) measured from top of footing.



Item 5: Height of the passive soil (ft) measured from top of footing.

Item 6: Height of the passive soil to be ignored (ft).

Item 7: Height of the stem (ft) measured from top of footing.

Item 8: Type of the stem. Options are: Reinforced Concrete and Reinforced Masonry (see Figure 3 for reinforced masonry).

Item 9a: Thickness of the RC stem (in.) at the top of the wall.

Item 10a: Additional thickness (in.) of tapered RC stem on the heel side of the wall at its base. Put "0" if the RC stem is of uniform thickness.

Item 11a: Additional thickness (in.) of a tapered RC stem on the toe side of the wall at its base. Put "0" if the RC stem is of uniform thickness.

| Stem | | | |
|---------------------------|----------------------|--------|--------|
| Height from top of footin | Ig | 9 | ft |
| Wall type | Reinforced M | asonry | \sim |
| Nominal width of CMU un | nits <mark>9b</mark> | 12 ~ | in. |
| Masonry weight 1 | 0b Medium V | Veight | \sim |
| Design method | 11b ASD | | \sim |

Figure 3. Stem GEOMETRY input panel for Reinforced Masonry Walls

Item 9b: Nominal width of CMU stem (in.). Options are: 6, 8, 10, 12, 14, and 16 in. wide units.

Item 10b: Weight class for the CMU stem (in.). Options are: Normal Weight, Medium weight, and Light Weight.

Item 11b: Design method for the CMU stem (in.). Options are: Allowable Stress Design (ASD) and Strength Design.

| | To Restraints | | _ | | × | |
|----|-----------------------|-------------|---------------|----------|-------------------|----|
| | Number of restraint: | along the l | height of the | e stem 1 | ~ <mark>12</mark> | la |
| 12 | Stem is pinned a | at the base | | | | |
| | Restraints location 1 | rom the top | o of the stem | <u>:</u> | | |
| | Restraint 1 at | 0.0 | ft 12c | | | |
| | Restraint 2 at | | ft | | | |
| | Restraint 3 at | | ft | | | |
| | Restraint 4 at | | ft | | | |
| | Restraint 5 at | | ft | | | |
| | | Done | | | | |

Figure 4. Restraints input window



Item 12: Select if the wall is restrained.

Item 12a: determine number of restraints along the height of the stem.

Item 12b: Select if the stem is pinned at the base.

Item 12c: determine the location of restraint point(s) measured from the top of the stem.

Item 13: Thickness of the footing (in.).

Item 14: Width of the heel (ft).

Item 15: Width of the toe (ft).

Item 16: Select if shear key is provided.

Item 17: Width of the shear key (in.).

Item 18: Depth of the shear key (in.).

Item 19: The shear key's position. Options are: Under the stem, Heel side, Toe side, and "Distance from tip of the toe". If the last option is selected, the shear key's location must be specified in Item 20.

Item 20: Location (distance) of the shear key from the tip of the toe (ft).

Item 21: Select if passive pressure applies on the shear key only (neglecting passive pressure on the side of the footing).

Item 22: Select to adjust active-side pressure on the shear key. If left unselected, the active pressure distribution from above the shear key is extended up to the bottom of the key, which might be too conservative.

Item 23: User-adjusted factored pressure at the top of the shear key (psf/ft).

Item 24: User-adjusted factored pressure at the bottom of the shear key (psf/ft).



MATERIALS

| Basics Geo | ometry Materi | als Loads | s Reinf | orcement |
|---------------------------------------|--|-----------|--|----------|
| Backfill Soi | il . (c) | | | |
| Unit weigh | t (<u>γ</u>) | | 110 1 | pcf |
| Analysis ty | pe | | | |
| Equivalent | nuid density | | 50 5 | pst/ft |
| Angle of e | xternal friction (| δ) ? | 20 4 | degrees |
| - Soil Over T | oe | | | |
| Unit weigh | t <u>(γ)</u> | | | pcf |
| Analysis ty | pe | | | |
| (for passiv | riula density e soil) | 6 | 50 7 | pst/tt |
| Soil Benea Allowable Footing-so | th the Footing bearing pressur bil friction coeffi | e [| 3000 <mark>8</mark> 0.35 <mark>9</mark> | psf |
| Water Tabl | e ? | _ | | _ |
| Water unit | weight | e | 52.4 <mark>10</mark> | pcf |
| Backfill soi | il saturated γ | 1 | 130 <mark>11</mark> | pcf |
| Backfill soi | il saturated ϕ | 3 | 30 <mark>12</mark> | degrees |
| Structure | | | | |
| Concrete u | unit weight | 1 | 150 13a | pcf |
| Stem conc | rete f'c | 4 | 4000 14 a | psi |
| Footing co | oncrete f'c | 3 | 3000 <mark>15</mark> | psi |
| Rebar fy | | 6 | 50000 <mark>16</mark> | psi |

Figure 5. MATERIALS input page of RTWall

Item 1: Backfill soil's unit weight (pcf).

Item 2: Method of backfill pressure analysis. Options are: Rankine theory method (Rankine), Coulomb theory method (Coulomb), and Equivalent Fluid Pressure (EFP) method.

Item 3: Equivalent fluid density of the backfill soil (psf/ft) when EFP is selected for backfill pressure analysis.



Note: As shown in Figure 6, when Rankine or Coulomb method is selected for backfill pressure analysis, Item 3 will be replaced by **Item 3a**, angle of internal friction of the backfill soil (degrees), and for Rankine method only, **Item 3b**, backfill soil's cohesion (psf).

| Backfill Soil | |
|---|---------------|
| Unit weight (γ) | 110 pcf |
| Analysis type | Rankine 🗸 |
| Angle of internal frictio (ϕ) | 30 3a degrees |
| Cohesion (c) | 0 3b psf |
| Angle of external friction (δ) ? | 20 degrees |
| Soil Over Toe | |
| Unit weight (γ) | 100 pcf |
| Analysis type | Coulomb ~ |
| Angle of internal frictioi (φ) | 30 7a degrees |
| | |

Figure 6. Different analysis type options

Item 4: Angle of external friction of the backfill soil (degrees).

Item 5: Passive soil's unit weight (pcf).

Item 6: Method of passive pressure analysis. Options are: Rankine theory method (Rankine), Coulomb theory method (Coulomb), and Equivalent Fluid Pressure (EFP) method.

Item 7: Equivalent fluid density of the passive soil (psf/ft) when EFP is selected for passive pressure analysis.

Note: As shown in Figure 6, when Rankine or Coulomb method is selected for passive pressure analysis, Item 7 will be replaced by **Item 7a**, angle of internal friction of the backfill soil (degrees), and for Rankine method only, another input field for backfill soil's cohesion (psf).

- Item 8: Allowable bearing pressure of the soil beneath the footing (psf).
- Item 9: Footing-soil friction coefficient.
- Item 10: Water's unit weight (pcf).
- Item 11: Backfill soil's saturated unit weight (pcf).
- Item 12: Backfill soil's saturated angle of internal friction (degrees).
- Item 13a: Concrete's unit weight (pcf).

Item 14a: Specified compressive strength of the RC stem concrete (psi).



| Structure | |
|----------------------|----------------|
| Stem rebar Grade | 13b Grade 60 V |
| Stem masonry f'm | 1500 14b psi |
| Footing concrete f'c | 3000 psi |
| Rebar fy | 60000 psi |

Figure 7. Structure MATERIALS input panel for Reinforced Masonry Walls

Item 13b: Rebar grade for the CMU stem. Options are: Grade 40, Grade 50, and Grade 60.

Item 14b: Specified compressive strength of the masonry material for the CMU stem (psi).

Item 15: Specified compressive strength of the footing concrete (psi).

Item 16: Yield strength of the structural steel rebar (psi).



LOADS

| Basics | Geometry | Materials | Loads | Reinforcement |
|----------------------|-----------------------------|---------------|--------------------|-------------------------------|
| Surch | arge (Unfacto | ored) | | |
| <mark>1</mark> 🔽 ປາ | niform surcha | rge | | |
| P | ressure 50 | 2 ps | f | |
| Other | surcharge: | 3 🗌 Strip | > 🗌 Lii | ne 🗌 Point |
| Dept | h (ft) | 4 0 | 0 | 0 |
| Dista of the | nce from top e stem (ft) | 5 5 | 5 | 5 |
| Widt | h (ft) | 2 6 | | |
| Magr (psf/ | nitude plf/lbs) | 7 100 | 100 | 100 |
| Conce | entrated Load | is at the To | p (Unfact | ored) |
| Axial | DL 0 8 | plf | Axial LL | 0 9 plf |
| Axial | load eccentri | city 0 10 | in. | |
| Latera | al loads: Wir | nd 11 | ~ | |
| Shea | ar 0 <u>12</u> | plf Mon | nent 0 | 13 lbs-ft/ft |
| Latera | al Loads (Unf | actored) | | |
| <mark>14</mark> 🗌 UI | niform lateral | pressure | Wind | 15 ~ |
| N | lagnitude | | 10 <mark>16</mark> | psf |
| S | tart point 0 | 17 ft | End poi | nt 2 18 ft |
| <mark>19</mark> 🗌 Se | eismic load | Nononobe- | Okabe Eq | uation $\frac{20}{\sqrt{20}}$ |
| s | DS 1.0 21 | g | | |
| к | h 0.4 22 | | Kv 0 | 0.0 23 |
| A | ngle of intern | al friction (| (φ) Β | ³⁰ 24 deg |

Figure 8. LOADS input page of RTWall

NOTE: All loads are to be entered unfactored.

- Item 1: Select if uniform surcharge (at the top of the backfill soil) exists.
- **Item 2:** Magnitude of the uniform surcharge (psf).
- Item 3: Select if strip, linear, and point surcharge (adjacent footings) exist.



Note: Applying a point (single) surcharge will result in designing the wall for the critical section along its length.

- Item 4: Depth (measured from the top of the backfill) of the strip, linear, and point surcharge (ft).
- Item 5: Distance from the top of the stem of the strip, linear, and point surcharge (ft), respectively.
- **Item 6:** Width of the strip surcharge (ft).
- Item 7: Magnitude of the strip, linear, and point surcharge (psf/plf/lbs).
- Item 8: Concentrated axial dead load at the top of the stem (plf).
- Item 9: Concentrated axial live load at the top of the stem (plf).
- Item 10: Eccentricity of axial loads from the center line of the stem (in.).

Note: Applying concentric moment (direct moment or eccentric axial loads) at the top of the stem requires vertical bars to be developed at the top – can be enabled under BASICS settings.

Item 11: Source of concentrated lateral loads (shear and moment) at the top of the stem. Options are: wind and seismic.

- Item 12: Concentrated shear at the top of the stem (plf).
- Item 13: Concentrated moment at the top of the stem (lbs-ft/ft).
- Item 14: Select if uniform lateral pressure acts on the stem.
- Item 15: Source of uniform lateral pressure on the stem. Options are: live, fluid, earth, and wind.
- Item 16: Magnitude of the uniform lateral pressure on the stem (psf).
- Item 17: Start point (measured from the top) of the uniform lateral pressure (ft).
- Item 18: End point (measured from the top) of the uniform lateral pressure (ft).
- Item 19: Select if seismic load applies on the wall.

Item 20: Method of seismic analysis. Options are: Mononobe-Okabe (M-O) Equation and Geotechnical Report Values.

Item 21: Value of the design short period spectral response acceleration, S_{DS} (g) when M-O equation is used for seismic analysis.

Item 22: Horizontal seismic acceleration factor when M-O equation is used for seismic analysis.

Item 23: Vertical seismic acceleration factor when M-O equation is used for seismic analysis.



Item 24: Angle of internal friction of the backfill soil (degrees). Required only when EFP is selected in the MATERIALS page for active pressure analysis.

Note: As shown in Figure 9, when using geotechnical report values is selected for seismic analysis, Items 21 through 25 will be replaced by **Item 21a**, strength-level seismic force (plf), and **Item 21b**, application point (distance from the top of the footing) of the seismic force (ft).

| Seismic load | Geotechnical Rep | ort Va | lues | ~ |
|----------------|----------------------|--------|------------|-----|
| Strength-Leve | el F _{seis} | 5000 | 21a | plf |
| App. point fro | om top of footing | 4.2 | 22b | ft |

Figure 9 Simplified seismic load input option



REINFORCEMENT

| B | asics | Geometry | Materials | Loads | Reinforcement |
|---|---------------------------|------------------|---------------|----------|---------------------|
| F | Stem | | | | |
| I | Longi | itudinal rein | forcement | | |
| I | Bar p | oosition Ce | entered layer | 1 ~ | |
| | Bar si Cove | ize #7 2 | fill) | Spacing | 12 <u>3</u> in. |
| | Embe | edment type | e Straight | 5 | ~ |
| l | Toe-s | ide layer of | bars | Bar size | #7 6 |
| l | Spaci | ing 12 7 | in. | Cover | 2 8 in. |
| 9 | ⊡ 1 | ransverse re | inforcement | | |
| l | Bar s | size #5 10 | | Spacing | 12 11 in. |
| F | Footi | ng | | | |
| I | Top r | einforcemer | <u>it</u> | Bar size | #7 12 |
| l | Spaci | ing 12 13 | in. | Cover | 2 <u>14</u> in. |
| l | Botto | om reinforce | ment | Bar size | # <mark>15</mark> ~ |
| l | Spaci | ing 12 <u>16</u> | in. | Cover | 3 <u>17</u> in. |
| 1 | <mark>8</mark> ⊿ <u>⊺</u> | ransverse re | inforcement | | |
| | Bar si | ize #5 19 | | Spacing | 12 20 in. |
| | | | | | |

Figure 10. REINFORCEMENT input page of RTWall

Item 1: Longitudinal reinforcement layout for RC stem. For an RC stem, the options are:

- One curtain (not available when the stem is restrained)
- Centered layer
- Two curtains.

For a masonry stem, the options are:



- Inner (backfill) edge (not available when the stem is restrained) One layer of reinforcement placed near the backfill face of the stem with a cover of 2 in.
- Outer edge (not available when the stem is restrained) One layer of reinforcement placed near the outer face of the stem with a cover of 2 in.
- Specified cover (not available when the stem is restrained) One layer of reinforcement placed with a cover specified in Item 4 shown below.
- Centered
- Two layers

Item 2: Bar size of the longitudinal reinforcement in the stem. When two curtains/layers of longitudinal reinforcement are provided, it is the bar size of the curtain/layer that is closest to the backfill. Options are: #3 through #18.

Item 3: Spacing of longitudinal reinforcement specified (in.) in Item 2.

Item 4: Concrete cover for longitudinal reinforcement (in.) specified in Item 2. Cover is specified from the backfill face of the stem.

Item 5: Type of embedment of the stem longitudinal reinforcement in the footing concrete. The options are: Straight, Hooked, and Check both. When "Check both" is selected, stem flexural strength is checked against the moment demand assuming both straight and hooked embedment.

Item 6: Bar size for stem's second layer of longitudinal reinforcement, if provided. Options are: #3 through #18.

Item 7 Spacing of longitudinal reinforcement specified (in.) in Item 6.

Item 8: Concrete cover for longitudinal reinforcement (in.) specified in Item 6. Cover is specified from the outer face of the stem.

Item 9: Select if the stem has transverse (shrinkage and temperature) reinforcement.

Item 10: Bar size for stem's transverse reinforcement, if provided. Options are: #3 through #18.

Item 11: Spacing of stem's transverse reinforcement (in.), if provided.

Item 12: Bar size for footing's top reinforcement. This is the longitudinal reinforcement for resisting flexure in the heel. Options are: #3 through #18.

Item 13: Spacing of footing's top (heel) reinforcement (in.).

Item 14: Concrete cover for footing's top (heel) reinforcement (in.).

Item 15: Bar size for footing's bottom (toe) reinforcement. This is the longitudinal reinforcement for resisting flexure in the toe. Options are: #3 through #18.

Item 16 Spacing of footing's bottom (toe) reinforcement (in.).

Item 17: Concrete cover for footing's bottom (toe) reinforcement (in.).



Item 18: Select if the footing has transverse (shrinkage and temperature) reinforcement.

Item 19: Bar size for footing's transverse reinforcement, if provided. Options are: #3 through #18.

Item 20: Spacing of footing's transverse reinforcement (in.), if provided.



CHECK DESIGN AND QUICK RESULTS

Upon completing design details input, user can click on the "Check!" button to verify if the design meets code's stability and structural requirements.

| | Factors of Safety | | |
|--------|---|------------------------------|--|
| | Stability Checks | | |
| | Overturning. | 2.56 | |
| | Sliding: | 0.67 | |
| | Bearing: | 1.81 | |
| | | | |
| | Utilization F | Ratios | |
| Q | Stem Design | | |
| I | Flexure: | 1.01 | |
| ĸ | Shear: | 0.32 | |
| > | Other*: | NG | |
| R | | | |
| E S | Heel Design | | |
| UL | Flexure: | 0.39 | |
| T | Shear: | 0.57 | |
| 1 | Other*: | ок | |
| | Toe Design | | |
| | Flexure: | 0.71 | |
| | Shear: | 0.72 | |
| | Other*: | ок | |
| | * Other checks inc - max. reinforcemen - min. reinforcemen - bar spacing | lude: nt ratio t ratio | |

Figure 11. Results bar after running the design check

RTWall provides user with two output options. The first option is checking quick results shown in the results bar (Figure 11) that shows whether the design complies with the code's requirements for stability (overturning, sliding, and bearing) and structural design (stem, heel, and toe). The second option is checking the detailed output through using the "View Report" button.



OUTPUT INTERFACE

The detailed output report (Figure 12) can be displayed by clicking on the "View Report" button.

| Prepared by: | |
|--|--|
| ~ | |
| Company: | |
| Phone: Email: | |
| Soil Over Toe: | |
| $\gamma = 100 \text{ pcf}$ | |
| Analysis type: EFP $\gamma_{EFP} = 50 \text{ psf/ft}$ | |
| Soil under the Footing: | |
| Allowable bearing pressure = 3000 psf | |
| Footing-soil friction coefficient = 0.35 | |
| Water: | |
| $\gamma_{\text{water}} = 62.4 \text{ pcf}$ | $\phi_{saturated} = 30^{\circ}$ |
| $\gamma_{saturated} = 130 \text{ pcf}$ | |
| Structure: | |
| $\gamma_{\text{concrete}} = 150 \text{ pcf}$ | Stem concrete $f_c = 4000 \text{ psi}$ |
| Footing concrete $f_c = 3000 \text{ psi}$ | $f_y = 50000 \text{ psi}$ |
| PROJECT INFORMATION - Loads | |
| Surcharge: | |
| Uniform surcharge pressure = 50 psf | |
| Lateral loads: | |
| Wind-type unifrom lateral pressure: magnitude = 10 p | psf, located at 0 to 6 ft from the top |
| Seismic load on backfill: $S_{DS} = 1 \text{ g}, K_h = 0.4, K_h$ | $v = 0, \phi = 30^{\circ}$ |
| PROJECT INFORMATION - Reinforcement | ł |
| Stem reinforcement: | |
| Longitudinal bars (main layer): #7 @ 12 in cover = | = 0 in. from backfill |
| Embedment type: Straight | |
| Transverse bars: #5 @ 12 in. | |
| Top reinforcement: | |
| Main bars: #7 @ 12 in cover = 2 in. from the top of | f the footing |
| Bottom reinforcement: | |
| Main bars: #7 $@$ 12 in cover = 3 in. from the bottom | m of the footing |
| Footing transverse reinforcement: | |
| #5 @ 12 in. | |

Figure 12. Output report